# CITY of NOVI CITY COUNCIL



Agenda Item F June 7, 2010

**SUBJECT:** Adoption of a Resolution seeking participation in the Michigan Department of Transportation (MDOT) Local Bridge Program for a grant application for the rehabilitation of the Cranbrooke Drive Bridge over Ingersol Creek.

SUBMITTING DEPARTMENT: Department of Public Services, Engineering Division

## CITY MANAGER APPROVAL

EXPENDITURE REQUIRED	\$ 12,400
AMOUNT BUDGETED	\$248,000 (Included in approved FY10-11 budget)
LINE ITEM NUMBER	204-204.00-865.942

#### BACKGROUND INFORMATION:

The Cranbrooke Drive bridge over Ingersol Creek (f/k/a as Courter Ditch) was inspected in November 2008 as required every two years by the state (see report dated November 19, 2008, attached). The inspection revealed that the bridge is in fair to poor condition and requires some rehabilitative maintenance work. The rehabilitation would include:

- Repair all delaminated/spalled concrete at each abutment;
- Replace damaged bearings;
- Repair beam ends; and,
- Repair slope paving and stabilize the area with rip rap to prevent future scouring.

It is important to note that the bridge is functional and is not in a condition that requires a closure or presents an immediate hazard to the public.

Engineering staff has prepared and submitted the enclosed application for funding under MDOT's 2010 Local Bridge Program. The total amount requested for the project is \$248,000 which includes \$205,000 for rehabilitative work and \$43,000 for engineering costs. A resolution from City Council is required as part of the application package. Selected projects will receive 95% funding with a local match of 5% for the 2013 fiscal year; however, there is an option to advance construct the project with City funds now and receive reimbursement later. If awarded the grant, the City would save \$235,600 (\$248,000 less a local match of \$12,400).

The project is included in the approved FY2010-11 Capital Improvement Program. The budgeted funds could be used to advance construct the project (with reimbursement in 2013) if a grant is awarded, or to complete the rehabilitation of the bridge if the City does not receive Local Bridge Program funding.

**RECOMMENDED ACTION:** Adoption of a Resolution seeking participation in the Michigan Department of Transportation (MDOT) Local Bridge Program for a grant application for the rehabilitation of the Cranbrooke Drive Bridge over Ingersol Creek.

	1	2	Y	Ν
Mayor Landry				
Mayor Pro Tem Gatt				
Council Member Crawford				ter tit horis hi
Council Member Fischer				

	1	2	Y	N
Council Member Margolis				
Council Member Mutch				
Council Member Staudt				



### RESOLUTION

WHEREAS,the Mayor and the City Council of the City of Novi are required<br/>under provisions of the Local Bridge Program to review, approve,<br/>and state that they are actively seeking participation in certain<br/>bridge rehabilitation projects; and,IndryWHEREAS,the staff of the City of Novi has reviewed the bridge system in Novi<br/>and found that there is a need for the rehabilitation of the<br/>Cranbrooke Drive bridge intersecting the Courter Ditch (Ingersol<br/>Creek) to enhance traffic safety and improve the bridge's<br/>structural capacity; and,

WHEREAS, the available funds are insufficient to fund the bridge project submitted while still maintaining and upgrading the remainder of the road system.

**NOW, THEREFORE, BE IT RESOLVED** that the Mayor and Novi City Council hereby seek participation in the Local Bridge Program for the following project and affirm a commitment to provide local funds in the amount of a 5% match in the event the project receives Federal and State funding.

#### Bridge and Location

Estimated Total Construction/Design Cost

\$248,000

Cranbrooke Drive over the Courter Ditch (Ingersol Creek)

### CERTIFICATION

I, Maryanne Cornelius, duly appointed City Clerk of the City of Novi; do hereby certify that the foregoing is a true and complete copy of a Resolution adopted by the City Council of the City of Novi at a Regular meeting held this 7th day of June, 2010.

Maryanne Cornelius City Clerk

CITY COUNCIL

Mayor David B. Landry

Mayor Pro Tem Bob Gatt

Terry K. Margolis

Andrew Mutch

Kathy Crawford

Dave Staudt

Justin Fischer

City Manager Clay J. Pearson

Director of Public Services/ City Engineer Rob Hayes

Department of Public Services Field Services Complex 26300 Delwal Drive Novi, Michigan 48375 248.735.5640 248.735.5659 fax

# **2010 Local Bridge Program Application**



Deadline: June 1, 2010

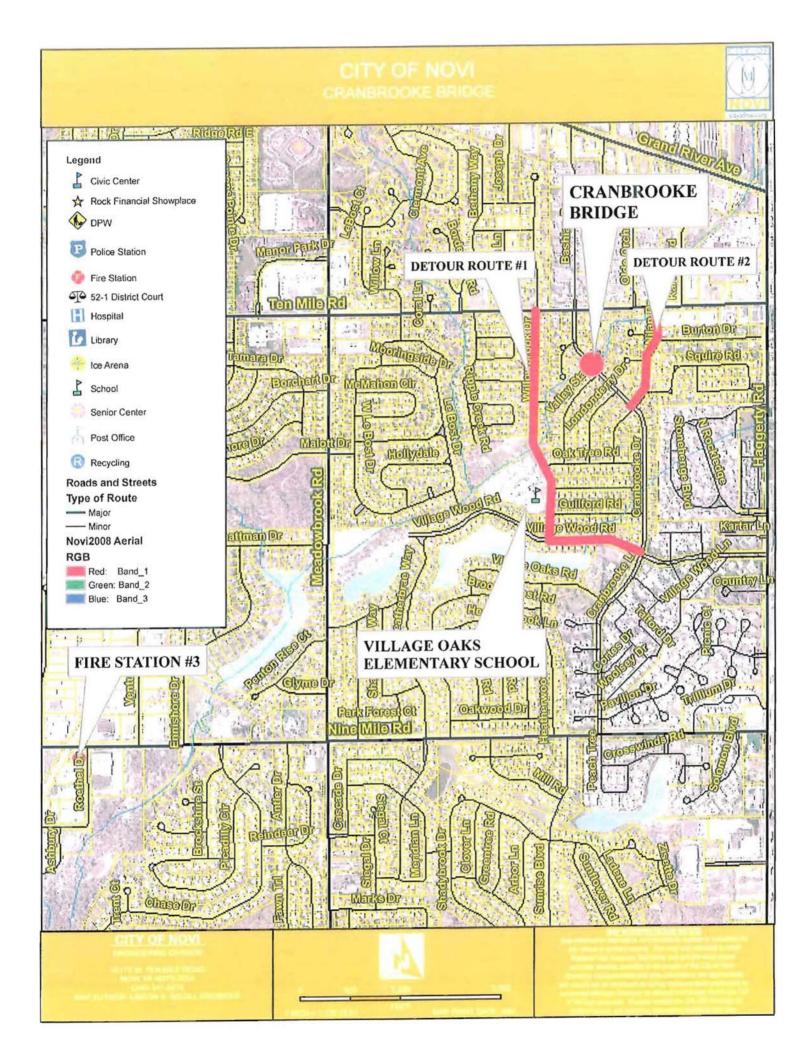
Applicant: City of Novi 45175 W. Ten Mile Road Novi, MI 48375

Contact: Brian T. Coburn, P.E., Senior Civil Engineer Phone: (248) 735-5632 bcoburn@cityofnovi.org

\*\*\*The resolution from our City Council has been placed on the June 7, 2010 City Council Agenda. The resolution shall be submitted no later than June 14, 2010.

### Index

- -Map of Area (including emergency facilities, schools and detour routes)
- -Application Request
- -Economic Importance
- -Detour Affect
- -Cost
- -Williams and Works Inspection Report Summary
- -Williams and Works Recommendation
- -MDOT Bridge Safety Inspection Report
- -MDOT Structural Inventory and Appraisal
- -MDOT Bridge Inspection Report
- -MDOT Level One Scour Analysis Worksheet
- -Photographs



#### **Application Request**

The City of Novi is submitting this application for rehabilitation of the bridge on Cranbrooke Drive spanning over the Courter Ditch. An inspection was completed by Williams & Works on behalf of the City of Novi in November of 2008. The inspection report concluded that the the structure is in fair to poor condition. The report stated that the joints between beams are leaking, there are several spalled areas including exposed reinforcement along the top flanges of beams, heavily corroded bearing plates, there is cracking at the bottom of concrete beams near the bearings, heavy water leaking from the backwall and undermined slope protection caused by scour among other issues stated in the inspection report. The rehabilitation would include:

- Repairing all delaminated/spalled concrete at each abutment
- Replaced damaged bearings
- Repairing beam ends
- Repairing the current slope protection and stabilizing the area with rip rap in order to prevent future scour

The Federal Sufficiency Rating Points rated the Cranbrooke Drive bridge structurally deficient with a 69.7 rating.

#### **Economic Importance**

Cranbrooke Drive is a residential collector that carries traffic to nearby arterials. Haggerty Road is located east of, and parallel to Cranbrooke Drive and is a heavily traveled corridor. Cranbrooke alleviates the neighborhood traffic from the Haggerty Road corridor.

#### **Detour Effect**

If the structure is closed, the detour would have an effect many adjacent residential roads. Cranbrooke Drive is a residential collector road linking Nine Mile Road to Ten Mile Road through densely populated subdivisions of Heathergreen, Fairfield Farms, Lakewood Park Homes and other adjacent subdivisions. The detour would inconveniently reroute many residents, especially those living on Cranbrooke Drive, through lower volume residential streets. A detour would also negatively impact bus routes as there is an elementary school nearby.

#### Cost

1) Right-of-Way	(1) \$0	
2) Design Engineering	(2) \$25,000	
3) Construction Engineering	(3) \$18,000	
TOTAL (1,2 & 3)	\$43,000	
A. Approach Construction	(A) \$0	
B. Structure Construction	(B) \$205,000	
TOTAL (A & B)	\$205,000	

All above costs include a 15% contingency in addition to what is shown on the Cost Estimate sheet (next page).

#### **Priority List**

1) Bridge on Cranbrooke Drive spanning the Courter Ditch

#### Resolution

As stated on the cover sheet, the resolution from our City Council has been placed on the June 7, 2010 City Council Agenda. The resolution shall be submitted no later than June 14, 2010.

#### Cost Estimate for Design & Reconstruction of the Cranbrooke Drive Bridge over Courtier Ditch

Structure Repair	Quantity 1	Unit LS	Unit Price \$10,000.00	<u>Cost</u> \$10,000.00
Removal/Replacement of Existing Landscaping in Median (across bridge) Preformed Waterproofing Membrane	720	Sft	\$10,000.00	\$2,160.00
Hot Poured Joint Sealer for Deck Joints	256	Ft	\$15.00	\$3,840.00
Excavation	80	Cyd	\$10.00	\$800.00
Structure Backfill	80	Cyd	\$13.00	\$1,040.00
oint Waterproofing	240	Sft	\$5.00	\$1,200.00
fand Chipping, Other Than Deck	148	Cft	\$40.00	\$5,920.00
Patch, Forming	295	Sft	\$40.00	\$7,375.00
	6	Cyd	\$1,000.00	\$6,000.00
Patching Conc, C-L	30	Ea	\$20.00	\$600.00
Idhesive Anchoring of Reinforcing Bars		Lb	\$20.00	
poxy Coated Steel Reinforcement	3230 396	Sft	\$150.00	\$4,845.00
land Chipping for Beam End Repairs	13	1.1.1.1.1.1		\$59,400.00
atching Concrete for Beam End Repairs	396	Cyd Sft	\$1,200.00 \$40.00	\$15,600.00
orming of Beam End Repair Patches	390	Sft	\$35.00	\$15,840.00
lastomeric Bearing Pad, 1 inch	35	LS	\$35.00	\$1,225.00 \$10,000.00
lope Protection Repair	1.			
iprap, Heavy	181	Syd	\$50.00	\$9,050.00
rosion Control Measures	1	LS	\$3,000.00	\$3,000.00
mbankment, CIP	20	Cyd	\$8.00	\$160.00
ubbase, CIP	52	Cyd	\$8.00	\$416.00
ggregale Base, Modified 6 inch	156	Syd	\$7.50	\$1,170.00
pproach Pavement - Non-Reinf Concrete	156	Syd	\$55.00	\$8,580.00
urf Establishment - Seed	150	Syd	\$5.00	\$750.00
		Structure S	ubtotal:	\$168,971.00
esign & Construction Engineering Costs (18%)		Total Engin	eering Cost:	\$30,414.78
ontingency (20%)	1	LS	\$33,794.20	\$33,794.00
		Total Cost:		\$233,200.00

Williams & Works

November 19, 2008

Mr. William McCusker DPW Director City of Novi 26300 Delwal Street Novi MI 48375

Re: 20

2008 Bi-Annual Bridge Inspections-Cranbrooke Drive over Courter Ditch

Dear Mr. McCusker:

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We have completed our biannual inspection of the Cranbrooke Drive structure over Courter Ditch. The enclosed inspection report includes the following items:

- Structure Inventory and Appraisal 1717A form
- Bridge Safety Inspection Report P2502 form
- Level 1 Scour Analysis
- Recommended solutions for identified problems (if any)
- Recommendations for preventative maintenance items (if any)
- Photographs.

In general the structure is in fair to poor condition. The joints between the beams are leaking, with efflorescence present, there are several spalled areas and exposed reinforcement along the top flange of the beams. This is worst at the center joint below the landscaped median of Cranbrook Drive. The toe of the west sidewalk is spalled with exposed reinforcement along nearly the entire length of the walk. The bearing plates are heavily corroded with pack rust evident at the sole plates and the elastomeric pads are cracked and bulging. The bottom of the concrete beams at each bearing is cracked approximately 1" above the bottom of the beam. This deteriorated area extends 6" to 1 ft from the the sole plate of the beam and was observed at each beam end at each abutment. Although no rebar was exposed, it is evident that the concrete bond with the reinforcing bars has been broken.

The abutment seat is delaminated and spalled at several locations along the north abutment. Evidence of heavy water leakage from the backwall is evident along the entire face of both abutments with rust staining and efflorescence present. The existing slope protection has been undermined by scour and has settled and cracked.

Williams & Works

In order to correct these problems, the following is recommended:

- Repair all delaminated/spalled concrete at each abutment
- Replace the bearings
- Repair the beam ends
- Repair the existing slope protection and place additional riprap below the bridge to prevent further scour.

One option for repairing the beam ends and bearings is to fully encase the beam ends in concrete.

Recommended preventative maintenance items include:

<sup>6</sup> Seal deck joints. This includes removing the landscaping in the median over the bridge and placing waterproofing on the bridge deck.

No plans detailing the foundation type of the existing bridge are available. For this reason, the SI&A Item 113 – Scour Criticality, has been updated to a code of "U" for unknown foundation type. For this reason, a Level 2 Scour analysis is recommended.

If you should have any questions or require additional information please don't hesitate to call. We appreciate the opportunity to be of service to you and look forward to working with you in the future.

Respectfully submitted,

Williams & Works, Inc.

Susan R. Tebbe, P.E.

Encl.

Facility				Federal Structure ID Inspector Name Agency/Consultant Inspection Date Legend
	DRIV	Ε		635489000049B01       Susan Tebbe       Williams & Works       [11/11/2008]       9       New         Latitude       Longitude       Struc Num       Insp Freq       Insp Key       7-8       Goo
Feature COURTER DIT	ĊH	•		42 27' 56.66" [83 26' 34.12" [8248 24 [QZSV]] 5.6 Fair
Location				Length Width Year Built Year Recon Br Type Scour Eval No.Pins 3-4 Pool
0.1 MI S OF TE				42 73.82 1974 5 04 U
		4 [(	06 0	
				DECK
1. Surface SIA-58A	8	8	7	Several transverse cracks in concrete surface in northbound lanes. Bituminous patch at longitudinal joint in southbound lanes. Landscaping across bridge in median. ( 08) Concrete pavement is in good condition. ( 06) ( 04)
2. Expansion Its	7		N	( 08) ( 06) ( 04)
8. Other loints				( 08) ( 06) ( 04)
. Railings	6	6	5	Concrete posts steel rails/pedstrian fencing. The last post of the pedestrian railing is damaged, with spalled concrete and exposed/bent reinforcing bars, in the NE quad. Posts and fence on bridge are in fair condition - no spalls to concrete or section loss in railing observed. (08) The last post of the pedestrian railing is damaged in the NE quadrant. Fence and post on bridge are in good condition. (06) (04)
. Sidewalks r curbs	7	7	6	The west sidewalk along the curb line is spalled and the rebar is visible. Ends of the rebar are visible along the curb line of the east sidewalk. (08)
,				The west sidewalk along the curb line is spalled and the rebar is visible. Ends of the rebar are visible along the curb line of the east sidewalk. ( 06) ( 04)
Deck ottom urface IA-58B				( 08) ( 06) ( 04)
Deck IA-58	6	6	6	Joints between beams show leavy leaking, efflorescence, and minor spalling. No exposed rebar observed. Bottom of concrete deck slabs not visible. (08) (06)
				small pieces of the deck or the stringers have broke off and are allowing dirt to come through in the grassy median onto the slope paving (04)
Drainage				Toe of sidewalk Is not cast on the bridge deck. Water allowed to drain from bridge from edge of roadway at the toe of sidewalk. No evidence of ponding on the bridge deck. (08) (06) (04)
				SUPERSTRUCTURE
perstructure A-59	6	6	5	Longitudinal cracks in the east fascia beam under the roadway. Top flange of this beam spalled and wet along 1/2 of the span length. Leaching and spalling between the beams observed. All beam ends are rust stained at bearings. The concrete at the bottom of the beam is cracked/spalled 1" deep x 6" to 1ft long at the bearings - typical for all beam ends at both abutments. (08) Longitudinal cracks in the east fascia beam under the roadway. Leaching and spalling between the beams. (06) (04)
Paint A-59A	Ν	N	N	(08) (06)

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Michigan Depa Form P2502	artment	of Tr	ansp	ortation Page Bridge Safety Inspection Report 6348905 0004900B0
Facility CRANBROOK	E DRIV	/E		Federal Structure ID Inspector Name         Agency/Consultant         Inspection Date         Legend           [635489000049B01]         [Susan Tebbe]         [Williams & Works         [11/11/2008]         9         New
Feature	TCH			Latitude         Longitude         Struc Num         Insp Freq         Insp Key         7-8         Good           42 27' 56.66"         83 26' 34.12"         8248         24         QZSV         5-6         Fair
Location 0.1 MI S OF TI	EN MILI	E RD		Length Width     Year Built Year Recon     Br Type     Scour Eval     No.Pins     3-4     Poor       42     73.82     1974     5     5     U     2 or Less Critical
		04 0		
11. Section Loss				( 08) ( 06) ( 04)
12. Bearings	6	6	4	All steel plates are heavily corroded with some pack rust present. The elastomer is bulging and cracked. The sole plates cast into the beams are also heavily corroded, some with extensive pack rust. Anchor bolts are heavily corroded. (08) The elastomer is bulging and craked. Steel plates are corroded. (06) bearing plates where the box beams rest are rusting and flaking off (04)
				SUBSTRUCTURE
13. Abutments SIA-60	7	6	4	North abutment seat spalled and delaminated at beams 4E thru 8E and 4W thru 6W. The spalled/delaminated areas extend under the bearings (5-10% of bearing area). The abutment walls are rust stained and efflorescence is present along the entire length of both abutments. 2' x 8" spall at the top of the south abutment. Rebar visible at several of the spalled areas. (08) Vertical cracks at the road drainage opening locations in all four quadrants. 2' x 6" spall at the top of the south abutment. Several horizontal leaching cracks 2'-3' long at the top of the abutment. Rebar visible in a few locations. (06) (04)
4. Piers iIA-60	N	N	N	( 08) ( 06) ( 04)
5. Slope rotection	5	5	5	Slope paving has been severely undermined and has settled and cracked. There is no toe header for the concrete slope paving. Animals have dug between the slope opaving and abutment wall at the north abutment. (08) Slope paving has been severely undermined and has settled, but few cracks. Animals have dug between the slope paving and abutment wall. (06) the slope paving is sliding into the channel ,there is no toe header at the slope paving and the channel (04)
				APPROACH
5. Approach avt	7	6	6	The approach pavement has settled 1/2" +/- in all quadrants. The concrete approach pavement has a few small areas of bituminous patching at the longitudinal joints. Few transverse cracks observed in the northbound lanes. (08) Has settled 1/2"+/- in all quadrants. (06) (04)
7. Approach hIdrs Swalks	7		,	There are no approach sidewalks present. ( 08) There are no sidewalks present. ( 06) ( 04)
. Approach opes				( 08) ( 06) ( 04)
. Utilities				No utilities attached to the bridge. (08) There is a cable that was draped from one wingwall to the other on the west side of the bridge. It appears to be a cable TV line that was not installed properly. (06) (04)
. Channel A-61	4	5	5	The channel is clear of debris, the banks are in fair condition with slight erosion of the channel bank. The slope paving beneath the bridge is undermined. (08) The channel is clear of debris, the banks are in good condition and there are no signs of scour. The slope paving is in fair condition (see comments above). (06) (04)

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Michigan Departme Form P2502	ent of Transporta	tion	Bridge Safety Inspe	ection Report	6348905	Page 3 0004900B01
Facility CRANBROOKE DF Feature COURTER DITCH	RIVE]	635489000049BC	ngitude – Struc Num	Agency/Consultant Williams & Works Insp Freq [24	Inspection Date 11/11/2008 Insp Key	Legend 9 New 7-8 Good 6-6 Fair
Location 0.1 MI S OF TEN M 21. Drainage	IILE RD	Length Width	Year Bullt Year Recon 1974 [] NBI INSPEC	Br Type Scour Eva 5 5 U		3-4 Poor 2 or Less Critical
CulverIs Guard Rail 36A 0 36B 0 36C 0 36C 0 36D 0	Crit Feat Insp( 92A Frac Crit 92B Und. Watr 92C Spl.Insp Fatg Sntv.Insp	Freq Date	71 Watr Adeq [8 72 Appr Align 8 Temp Supp Hi Ld Hit (M) Special Insp Equip.			

Form 1717A-01/2002 MDOT Bridge ID 6348905 0004900E			Michigan Department of Transportation Structure Inventory and Appraisal <u>6348905</u> 0						
NBI Bridge ID 635489000049B01	Struct Num	Region	TSC 7B	County	City Resp 4890	City Location	7- FacIlity Carried		
6- Feature Intersecte			10	Latitude	Longitude	Owner	Maint Resp		
COURTER DITCH	0.1 MI S	OF TEN M	LE RD	42 27' 56.66"	83 26' 34.12	4	4		

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Bridge History, Type, Materials	Route Carried By Structu	re(ON Record	) Route Under Structure(U)	DER Record)
27 - Year Built 1974	5A - Record Type	1	5A - Record Type	
106 - Year Reconstructed	5B - Route Signing	5	5B - Route Signing	
202 - Year Painted	5C - Level of Service	0	5C - Level of Service	
203 - Year Overlay	5D - Route Number	00000	5D - Route Number	· · · · · · · · · · · · · · · · · · ·
43 - Main Span Bridge Type 5 04	5E - Direction Suffix	0	5E - Direction Suffix	
44 - Appr Span Bridge Type	10L - Best 3m Uncir-Lt	99 99	10L - Best 3m Uncir-Lt	
77 - Steel Type 0	1 10R- Best 3m Unclr- Rt	99 99	10R- Best 3m Uncir- Rt	
78 - Paint Type 0	PR Number		PR Number	
79 - Rail Type 1	Control Section	0	Control Section	[]
80 - Post Type 0	11- Mile Point	0.0	11- Mile Point	[]]
107 - Deck Type 2	1 12- Base Highway Network	0	12- Base Highway Network	
108A - Wearing Surface 9	13- LRS Route-Subroute	000	13- LRS Route-Subroute	
108B - Membrane 8	19- Detour Length	2	19- Detour Length	
108C - Deck Protection 0	20- Toll Facility	3	20- Toll Facility	[]
	26- Functional Class	19	26- Functional Class	
Structure Dimensions	28A - Lanes On	2	28A - Lanes Under	<u> </u>
	29 - ADT	1300	29 - ADT	
34 - Skew 0	30 - Year of ADT	1992	30 - Year of ADT	
35 - Struct Flared	32- Appr Roadway Width	40.0	42B- Service Type Under	5
45 - Num Main Spans 1	32A/B - Ap Pvt Type/Width	6 40.0	47L - Left Horizontal Clear	· · · · · · · · · · · · · · · · · · ·
46 - Num Apprs Spans 0	42A- Service Type On	5	47R- Right Horizontal Clear	
48 - Max Span Length 38.7	47L - Left Horizontal Clear	19.7	54A - Left Feature	N
49 - Structure Length 42	47R-Right Horizontal Clear	19.4	54B- Left Underclearance	99 99
50A - Width Left Curb/SW 5.91	53- Min Vert Clr Ov Deck	99 (99	54C- Right Feature	N
50B - Width Right Curb/SW 5.91	100- STRAHNET	0	54D- Right Underclearance	99 99
33 - Median 2	102 - Traffic Direct	2	Under Clearance Year	00 00
51 - Width Curb to Curb 62.0	109 - Truck %	2	55A - Reference Feature	N
52 - Width Out to Out 73.82	110 - Truck Network	0	55B- Right Horiz Clearance	327.8
112 - NBIS Length Y	114 - Future ADT	1600	56- Left Horiz Clearance	0
	115 - Year Future ADT	2012	100-STRAHNET	<u> </u>
Inspection Data	Freeway	0	102 - Traffic Direct	
			109 - Truck %	
90 - Inspection Date 11/11/2008	Structure Apprai	sal	110 - Truck Network	
91 - Inspection Freq 24		-	114 - Future ADT	
92A - Frac Crit Reg/Freg	36A- Bridge Railing	0	115 - Year Future ADT	
93A - Frac Crit Insp Date	36B-Rail Transition	0	Freeway	
92B - Und Water Reg/Freg N	36C- Approach Rail	0		
93B - Und Water Insp Date	36D- Rail Termination	0	Proposed Improvm	ents
92C - Oth Spec Insp Req/F., N	67- Structure Evaluation	6	75 - Type of Work	
93C - Oth Spec Insp Date	68- Deck Geometry	[	76- Length of Improvement	
176A - Und Water Insp Met.	69- Underclearance		94- Bridge Cost	
58 - Deck Raling 6		8	95- Roadway Cost	
58A - Deck Surface Rtg 7.		8	96- Total Cost	
59 - Superstructure Rating 5	103- Temporary Structure		97- Year of Cost Estimate	
59A - Paint Rating N		U	Load Rating and Pos	sting
60 - Substructure Rating 4				6
61 - Channel Raling 5	Miscellaneous		41- Open, Posted, Closed	A
62 - Culvert Rating N			63- Oper Rtg Method	2
	37- Historical Significance	4	64F- Fed Rtg Method	2 32.7
Navigation Data	98A- Border Bridge State		64M- Mich Oper Rtg	9 77
	98B- Border Bridge %			2
38 - Navigation Control		N		32.7
39 - Vertical Clearance 0	EPA ID		170 m //	5
40 - Horizontal Clearance 0	Stay in Place Forms		141- Posted Loading	
111 - Pier Protection			195- Analysis ID	
116 - Lift Brdg Vert Clear			193- Overload Class	
		1		· /

Michi Form	gan Department of	Transportation	Bridge Inst	section Report		6348905	Page 1 0004900B01
Facil	ity NBROOKE DRIVE		eral Structure ID Inspector Na 189000049B01 j Susan Tebbe			pection Date	
Featu COUI	Ire RTER DITCH	Latin] [42.2	tude Longitude Struc M 7' 56.66" (83 26' 34.12" (8248	Num Insp.Freq		isp Key IZSV	
Locat 0.1 M	tion ISOFTEN MILE R		gth Width Year Built Year Re 73.82 1974	econ BrType Sc 5 5 U	our Eval N	o.Pins	
		THERE AR	E NO CoRe ELEMENTS	5 FOR THIS ST	RUCTUF	E	
			WORK RECOMME	NDATIONS			
Γ	CR	EW RECOMMEN			ACT RECO	MMENDATION	VS
	Deck Patching			Bridge Replacement			
	Approach Pavement	L	Seal approach pavement joints.	Superstructure Replacement			
	Joint Repair	н	Seal joints on bridge deck. Remove landscaping in median and place waterproofing on bridge deck.	Deck Replaceme	nt		
	Railing Repair			Overlay			
	Detailed Insp			Widen			
	Zone Paint			Paint			
	Substr. Repair			Zone Paint			×.
	Slope Repair	н	Reapir existing slope paving and add additional riprap.	Pin and Hanger			
	Brush Cut			Substructure Repair	М	Repair spalle delaminated of both abutmen	concrete on
	Olher Crew Work			Other Contract Work	Н	Bearing replace and beam end	

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#### MICHIGAN DEPARTMENT OF TRANSPORTATION LEVEL ONE SCOUR ANALYSIS WORKSHEET

Date: 11/12/08 By: Williams & Works Structure No: B01 Control Section: 635489
Job No.: Route: Cranbrooke Drive Watercourse: Courter Ditch
All references are to HEC-20, 3rd Edition.
Data Collection         N/A       Plans (None available)         X       Bridge Inspection Reports (Maintenance Division)         N/A       Underwater Inspection Reports (Maintenance Division)         X       Review existing Items 60, 61, 71, 92, 93, and 113 of the NBIS         X       Review available construction, design, and maintenance files for repair and maintenance work done on structure
Field Investigation Date: 11/11/08
Channel bottom width approximately one bridge span upstream =12_feet
<u>X</u> Overbank and channel Manning's roughness coefficients <u>0.10</u> Left <u>0.035</u> Channel <u>0.10</u> Right
X_Is there sufficient riprap? Abutments N Piers N/A
X_Photographs
Cross sections at upstream and downstream faces of bridge
Comments:
Stream Characteristics
X Complete the attached Figure 2.6 from HEC-20.
Comments:
Land Use: Identify the existing and past land use of the upstream watershed:
Urban Area       Yes_X_No       Comments: City of Novi         Sand and Gravel Mining       YesNo_X       Comments:         Undeveloped Land       YesNo_X       Comments:

Lateral Stability: Refer to HEC-20, Section 2.3.9 on Channel Boundaries and Vegetation for channel bank stability. Comment: Banks are well established with well vegetated point bars.

Vertical Stability:

- streambed elevation change from as-built plans? UNKNOWN	Yes	No
- exposed pier footings (degradation)?	Yes	No <u>X</u>
- exposed abutment footings (degradation)?	Yes	NoX
- channel bank caving in (degradation)?	Yes	No <u>X</u>
- eroding floodplain (aggradation)?	Yes	No <u>X</u>
- crossing at confluence or tributaries?	Yes	No <u>X</u>
- bridge sites upstream and downstream?	Yes <u>X</u>	No
<ul> <li>grade or hydraulic controls, i.e., dams, weirs, diversions?</li> </ul>	Yes	No <u>X</u>
- foundation on rock	Yes	No <u>X</u>
- channel armoring potential	Yes	No_X_

Comments:

**Stream Stability:** Make a qualitative assessment of the overall stream stability by referring to the above information and Figure 2.6 and Table 3.2 from HEC-20 (attach copies of figures).

Stable\_\_\_\_ Unstable\_\_\_\_\_ Degrading\_\_\_\_\_ Aggrading\_\_\_\_\_

Comments: The existing slope protection at the bridge has been undermined due to scour.

#### RECOMMENDED NBIS ITEM 113 CODE: U

LEVEL TWO ANALYSIS NEEDED: YES X NO

Worksheet approved by: Juan Telffe P.E. License # 45698 Date 11/13/08

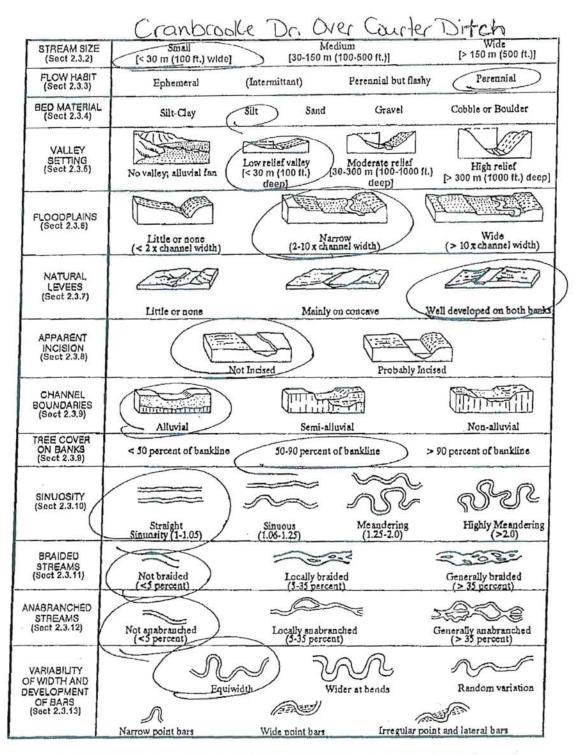


Figure 2.6. Geomorphic factors that affect stream stability (adapted from Brice and Blodgett).<sup>(10)</sup>



Cranbrooke Drive over Courter Ditch - East Fascia shown



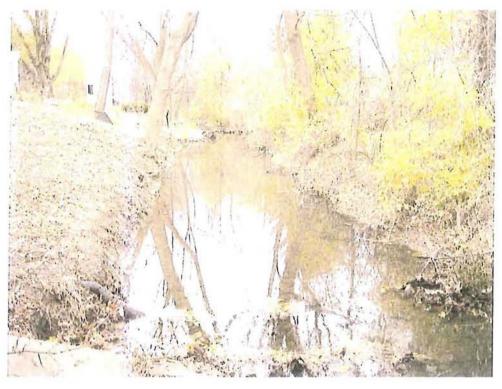
Typical condition of deck surface - Northbound lanes shown



Typical Approach Pavement Condition



Courter Ditch - Looking Downstream from structure



Courter Ditch - Looking Upstream from structure



Toe of west sidewalk – spalled concrete, expose and corroded reinforcing steel.



Toe of west sidewalk and top of abutment wall – spalled concrete with exposed reinforcement.



Damaged concrete post in the northeast quadrant of the bridge.



Typical condition of joints between beams – Top flange is wet with efflorescence and rust staining.



Typical condition of bearings. Bearings have heavy pack rust and section loss. Concrete at bottom of beam at bearings is cracked.



North Abut. Seat below beams 3W-6W wet. Beam seat is spalled between beams with exposed rebar. Spalls extend under bearing areas approx. 5%.



North Abut. Typical condition between beams 4E thru 8E. Abutment is wet, seat is spalled and rust stained.



North abutment and slope protection. Slope paving is undermined, settled, and cracked throughout.



South abutment and slope protection. Slope paving is undermined, settled, and cracked throughout.